



2011 - Chester VT 103 Bridge 8

Description

Meta Fields

Project Summary : Adjacent double T (NEXT) beams with precast abutments

Project Location : Vermont Route 103 (rural principal arterial) over the South Branch of the Williams River in the town of Chester in Windsor County

Mobility Impact Time : ABC: 3 weeks; Conventional: 6-month closure; 2 construction seasons

Impact Category : Tier 4 (within 1 month)

Dimensions : 56-ft long and 39.83-ft wide single-span prestressed concrete double-tee NEXT beam bridge

Average Daily Traffic At Time Of Construction : 7200

Traffic Management :

If constructed conventionally, extended use of 9-mile detour

Existing Bridge Description :

The existing single-span steel beam bridge was 46-ft long and 25-ft wide with cantilevered concrete abutments founded on spread footing. It had two 10.33-ft-wide traffic lanes and a 3.58-ft-wide sidewalk. Built in 1924, the bridge was deteriorated and required replacement.

Replacement Or New Bridge :

The Vermont Agency of Transportation (VTrans) reviewed the various construction options to limit traffic disruption during the replacement of Bridge 8 and a second bridge on VT Route 103, Bridge 9. The solution was to combine the two bridge replacements into one contract, detour traffic and close both bridges, use prefabrication to limit the closure time, specify incentives / disincentives in the contract to ensure the closure times were achieved, and do extensive public outreach prior to construction to inform the public of the impending closures. The Bridge 8 details are described here; see [Chester VT 103 Bridge 9](#) for details on the other bridge replacement in the project.

The replacement bridge has two 11-ft-wide traffic lanes, a 4-ft-wide shoulder and 5.5-ft-wide raised sidewalk on one side, and a 6-ft-wide shoulder on the other side. The cross-section consists of four 9.46-ft-wide, 28-inch-deep adjacent pretensioned concrete double T NEXT 28D beams spaced at 10.13-ft with an 8-inch-wide joint connecting the 8-inch-thick deck flanges. The outside beams were cast with 3-inch-thick raised sidewalk or curb. The 45.83-ft-long precast semi-integral abutments were each fabricated in three segments to reduce weight. Prefabricated elements also included precast approach slabs.

Construction Method :

Precast elements were fabricated with high-performance concrete (HPC) at a precast plant and shipped to the site. The two 3.5-ft-wide precast cap segments for Abutment 1 were match cast and fabricated with 2.0-ft x 2.5-ft pockets extending into the caps 3.33 ft from the bottom surface; 3-inch-diameter ducts extended from the top of the pockets to the cap surface. The contractor proposed a field change to the Abutment 2 details; VTrans accepted the proposal. As a result, three 2.0-ft-wide precast abutment stems for Abutment 2 were cast to the precast footing in the fabrication plant, to be placed on a cast-in-place concrete sub-footing that levels out the bedrock, and to be connected to the sub-footing with #8 dowel reinforcing bars inserted into a sleeve and filled with high-strength grout. The connecting ends of the precast Abutment 2 stems were keyed, providing a 1-inch-wide grout joint. The NEXT beams were cast with curtain walls at their ends.

The contractor detoured traffic and closed the bridge. At Abutment 1 the foundation was prepared and graded to the required elevation, and crews drove the 15-ft-long steel H-piles. The caps were erected over the piles, with piles embedded 3 ft minimum. The vertical match-cast key between segments was coated with a thin layer of epoxy bonding compound; the post-tensioning strands were installed in the ducts connecting the caps and tensioned; the ducts were not grouted. The cap pockets were filled with self-consolidating concrete. Backfill was then placed.

At Abutment 2 the contractor prepared the bedrock and placed the cast-in-place sub-footing. The precast cap/footing segments were installed on the sub-footing and the grade was established using leveling screws. After connecting the segments by filling the vertical keyways with grout, high-performance grout was used to fill the space between the precast footings and the sub-footing. Holes were drilled into the sub-footing through the sleeves provided in the precast footing segments and #8 dowel reinforcing bars were used to connect these components. The backfill was placed.

The beams were erected onto steel-reinforced elastomeric bearing pads on the abutment caps. The 8-inch-wide deck closure joints, with headed reinforcement bars extending from the beam flanges, were grouted using type 4 mortar expanded with 3/8-inch-diameter aggregate. The grade at the ends of the bridge was prepared, and the precast approach slabs were erected. Sheet membrane waterproofing and a 1.5-inch-thick asphalt overlay were placed. Longitudinal temporary barrier was placed to allow the bridge to be opened to two-way traffic. After the bridge was opened to traffic the sidewalk and aesthetic traffic railing were cast. Then the temporary barrier was removed and the final lift of 1.5-inch-thick

