



2006 - Mackey Bridge (Marsh Rainbow Arch Bridge)

Description

Meta Fields

Project Summary : This bridge was constructed with precast high performance concrete (HPC) full-depth deck panels, integral abutment footings, and interior bent pile caps.

Project Location : 120th Street over Squaw Creek north of the city of Des Moines

Mobility Impact Time :

- ABC: 6 months
- Conventional: 9 months

Impact Category : Tier 6 (longer but reduced by months/years)

Dimensions : 151.33-ft-long and 33.17-ft-wide three-span prestressed concrete I-beam bridge (47.4 ft $\times$ 56.5 ft $\times$ 47.4 ft); 30° skew

Average Daily Traffic At Time Of Construction : 30

Traffic Management : extended use of 4-mile detour

Existing Bridge Description : The existing two-lane single-span concrete Marsh arch bridge was 76 ft long and 18 ft wide roadway. Built in 1917, it was deteriorated and required replacement.

Replacement Or New Bridge : The replacement bridge has two 12-ft-wide traffic lanes and two 3-ft-wide shoulders. The superstructure cross-section consists of four 32-inch-deep prestressed I-beams (SLXA46 $\times$ end spans; SLXA55 $\times$ middle spans) at 8.33 ft spacing and an 8.25-inch-thick full-depth precast deck. The skewed 8-ft-long, 18.6-ft-wide transversely pretensioned deck panels span half the width of the bridge, joined by a 1-ft-wide longitudinal cast-in-place construction joint. The 32 interior panels were identical, and the four end panels had post-tensioning anchorage zones that required two unique panels at each end because the bridge was skewed. The substructure consisted of 3.0-ft-wide $\times$ 3.5-ft-deep precast integral abutment footings supported on steel H piles, and 3.2-ft-wide $\times$ 3.0-ft-deep precast pile caps supported on 16-inch-diameter steel pipe piles for the interior supports. High performance concrete (HPC) was used for all precast concrete on the job.

Construction Method : All precast elements were fabricated in the precast plant. Each panel was pretensioned in the transverse direction and had two 10-inch-wide full-depth channels over the beams for longitudinal post-tensioning. Although this design had not previously been constructed, it was developed from the NUDECK precast panel system, a similar full-depth deck system on steel girders that had been constructed and tested in Nebraska. The precast reinforced concrete abutment footings and pile caps were cast with full-depth pockets to go over the piles; the pockets were created using 21-inch-diameter corrugated metal pipe. The connection design was validated by testing. The piles were driven. The abutment footings were set in place over the H piles in less than 30 minutes. Similarly, the pile caps were placed over the pipe piles at cap pocket locations. A high-early-strength concrete mix was used to fill the pockets. The beams were erected onto the abutments and piers. The panels were erected onto the beams and leveled. The transverse joints were filled with concrete and allowed to cure overnight. Twelve post-tensioning strands were then placed in two layers in each longitudinal channel over the beams and stressed. The four post-tensioned channels over the beams, the longitudinal joint at the center of the bridge, and the abutment diaphragms were then cast. A pre-set mechanical splice system was used for barrier rail post connections. Surface profile grooving and grinding were done, although the deck was smooth enough for a gravel road without it. No overlay was applied.

Construction Timeline The contract did not force acceleration in terms of specified working days allowed with associated penalties since this project was the first ABC project in Iowa and in consideration of the limited budget (owned by the county) and the fact that other related work such as grading may have controlled the schedule. Construction began in July 2006 and took 108 working days to complete. Demolition of the previous bridge was completed in one day. Rain delayed the construction progress at the end of August and through most of September; in total, 21 working days were lost due to rain or wet site conditions. The piles were driven in late September and the precast substructure was set in early October; the girders and deck panels were set in late October after the substructure was complete. Transverse concrete joints were placed in late October and early November, during which time the post-tensioning tendons were placed and formwork for the post-tensioning channels and longitudinal center joint was installed. Construction was delayed again due to cold weather effects on the concrete joints; therefore, the joints had to be heated to ensure the

